



Simulations numériques pour la dynamique Réduction de modèle

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Formation Doctorale Vishno

A few activities



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Why does SDTools exist?



CAD/Meshing FEM Simulation Testing CATIA, Workbench, ...

NASTRAN, ABAQUS, ANSYS,... Adams, Simpack, Simulink,...

Siemens TestLab, ME-Scope, ...

- Necessity: programmatic access to all steps
- Chosen solution: flexible toolbox & custom app
 - Experimental Modal Analysis
 - Test / Analysis correlation
 - 3D Finite Element Modeling
- With a modular approach
 - MATLAB environment
 - OpenFEM : Core software for Finite Element Modeling (co-developed with INRIA)
 - FEMlink : import / export industrial modules
 - Runtime SDT : customized and standalone compiled applic



What is a system?



- Inputs u(t): hammer with force measurement
- Outputs y(t)
 - Test : vibrometer on testbed
 - Computation : stresses
- State x(t)
 - Displacement & velocity field as function of time
 - ${\dot{x}(t)} = f(x(t), u(t), p, t)$ evolution
 - $\{y(t)\} = g(x(t), u(t), p, t)$ observation
- Environment variables p
 - Dimensions, test piece (design point)
 - Temperature (value of constitutive law or state of thermoviscoelastic)
- Feature : function of output (example modal frequency)







 $_{\ensuremath{4}}$ Simple example : modified Oberst test for 3D weaved composite test

System models : nature & objectives



What is a model

- A function relating input and outputs
- For one or many parametric configurations

Model categories

- Behavior models (meta-models)
 - Test, constitutive laws, Neural networks
 - Difficulties : choice of parametrization, domain of validity
- Knowledge models
 - Physical principles, low level meta-models

Why do we need system models ? Design

- Become predictive : understand, know limitations
- Perform sizing, optimize, deal with robustness

<u>Certify</u>

- Optimize tests : number, conditions
- Understand relation between real conditions and certification
- Account for variability

Maintain during life

- Design full life cycle (plan maintenance)
- Use data for conditional maintenance (SHM)

Equations of motion

- Nominal model (elastic + viscous damping) $[M]{\ddot{q}} + [C]{\dot{q}} + [K(q)]{q} = [b(q)]{u(t)}$ $\{q\} \text{ DOF, M mass, K stiffness}$
- Loads decomposed as spatially unit loads and inputs ${F(+)} = [b] {u(+)}$
- {y} outputs are linearly related to DOFs {q} using an observation equation
 {y(t)} = [c] {q(t)}
- Simple case : extraction {w₂}=[0 0 1 0]{q}

6



 More general : intermediate points, reactions, strains, stresses, ...

Equations of motion

$\mathsf{FEM} \Leftrightarrow \mathsf{Reduction}$

	Finite elements	Reduction
	Continuous \rightarrow discrete full	Full \rightarrow reduced
Support	Element: line, tria, tetra,	FE mesh
Variable separ.	$w(x,t) = N_i(x)q_i(t)$	$\{q(t)\} = \{T_i\}q_i(t)$
Shape functions	$\epsilon(x,t) = B_i(x)q_i(t)$	T_i simple FE solutions
Matrix comp.	$K_{ii} = \int_{\Omega} B_i^T \Lambda B_i = \sum_{a} B_i^T (g) \Lambda B_i w_a I_a$	$K_{ijR} = T_i^T K T_j$
Weak form	numerical integration	FEM matrix projection
Assembly	Localization matrix	Boundary continuity, CMS
Validity	Fine mesh for solution gradients	Good basis for considered loading

- [1] O. C. Zienkiewicz et R. L. Taylor, The Finite Element Method. MacGraw-Hill, 1989
- [2] J. L. Batoz et G. Dhatt, Modélisation des Structures par Éléments Finis. Hermès, Paris, 1990
- [3] K. J. Bathe, Finite Element Procedures in Engineering Analysis. Prentice-Hall Inc., Englewood Cliffs, NJ, 1982

Ritz/Galerkin reduction from full

- Basis building steps
 - FEM : cinematically admissible subspace, virtual work principle
 - Reduction : 1) learn, 2) generate basis 3) choose DOF $\{q(p,t)\}_N \approx [T]_{N \times NR} \{q_R(p,t)\}_{NR}$
- Virtual work principle / reduction / Ritz-Galerkin Matrices $[M_R(p)] = T^T M(p)T, K_R(p) = T^T K(p)T$ Loads $\{f(p,t)\} = [b_R(p)]\{u(t)\} = [T^T b]\{u\}$ Observations $\{y(p,t)\} = [c_R(p)]\{q_R(p,t)\} = [cT]\{q_R\}$
- Solve time/freq (same model form) $[M_R]\{\dot{q_R}\} + [C_R]\{\dot{q_R}\} + [K_R]\{q_R\} = [b]\{u(t)\}$ $\{y(t,p)\} = [c_R]\{q_R\}$

Outline : solvers for dynamics

Continuous/discrete/reduced models (a brief reminder) Full order model solvers

- Direct frequency resolution
- Direct time integration (implicit/explicit, first/second order, Newmark, ... Gaël Chevallier)

Reduced order model + time/frequency resolution

- Basic reduction : modal superposition, static correction, Guyan, Craig-Bampton, ...
- Modern vision of reduction: learning phase, basis building, DOF choice
- Substructuring
- Parametric model reduction, error control

When does reduction become useful? Basic building blocks?

MATLAB Tutorial : direct frequency response issues

- Step1 : assembly, sparse matrices
- Step 2 : point load, collocated displacement, factorization strategies
- Step 3 : subspace around resonance, phase collinearity, SVD
- Step 4 : Rayleigh-Ritz, reduced FRF

Direct frequency response : Zq=F (step2)

- 1. Renumbering (fill in reduction, symbolic factorization, METIS, symrcm, ...)
- 2. Numerical factorization Z = LU or $Z = LDL^T$
- 3. Forward/backward solve $L(D(L^Tq)) = F$



Sparse librairies : Umfpack (lu), MA57 (ldl), Pardiso, Mumps, BCS-Lib, Spooles, Taucs, ...

Transfers : what subspace is needed ?



• Lower residual (rigid body inertia, ...)

Nearby modes = poor representation of static

Normal modes of elastic structure

Nominal model (elastic + viscous damping)

$$\begin{bmatrix} Ms^2 + Cs + K \end{bmatrix} \{q(s)\} = [b]\{u(s)\} \\ \{y(s)\} = [c]\{q(s)\}$$

Conservative eigenvalue problem

$$-[M] \{\phi_j\} \,\omega_j^2 + [K]_{N \times N} \{\phi_j\}_{N \times 1} = \{0\}_{N \times 1}$$

- M>0 & K \ge 0 \Rightarrow ϕ real
- Partial solvers exist

Normal modes of elastic structure

- Orthogonality
- Scaling conditions
 - Unit mass

 $[\phi]^T[M][\phi] = \left[{}^{\backslash} \mu_{j_{\backslash}} \right]$

$$\left[\phi\right]^{T}\left[K\right]\left[\phi\right] = \left[{}^{\backslash}\mu_{j}\omega_{j}^{2}{}_{\backslash} \right]$$

$$\{\phi_j\}^T [M] \{\phi_j\} = 1$$

- Unit amplitude $[c_s]{\{\tilde{\phi}_j\}} = 1$ $\mu_j(c_s) = ([c_i]\{\phi_j\})^{-2}$
- Principal coordinates

$$\left[[I]s^2 + [\Gamma]s + \left[{}^{\backslash}\omega_{j}^2 \right] \right] \{ p(s) \} = \left[\phi^T b \right] \{ u(s) \}$$

$$\{y(s)\} = [c\phi]\{p(s)\}$$

Modal contributions & static correction



Reduction <-> Ritz analysis

Response is approximated

$$q(s) = \left[\phi_1 \dots \phi_{NM} \quad [K_{Flex}]^{-1} [b]\right]_{N \times (NM + NA)} \left\{ \begin{array}{c} \vdots \\ \frac{\phi_j^T b u}{s^2 + \omega_j^2} \\ \vdots \\ \vdots \\ \end{array} \right\}$$

within subspace containing modes and flexibility

$$T = \left[\phi_1 \dots \phi_{NR} \quad \left[K_{Flex}\right]^{-1} \left[b\right]\right]$$

or modes and residual flexibility

$$T = \begin{bmatrix} \phi_{1:NM} & \left[K_{Flex} \right]^{-1} \begin{bmatrix} b - M \left[\phi_{1:NM} \right] \left[\phi_{1:NM}^T b \end{bmatrix} \end{bmatrix} \end{bmatrix}$$

Attachment modes



deformation in a uniformly accelerating frame

$$\{q_F\} = [K]_{Flex}^{-1}[b] = \sum_{j=NB+1}^{N} \frac{\{\phi_j\}\{\phi_j^T b\}}{\omega_j^2}$$

Unit imposed displacement

Applied load : free modes + static correction = McNeal Applied displacement : dynamic & Static/Guyan condensation

$$\begin{bmatrix} K_{II} & K_{IC} \\ K_{CI} & K_{CC} \end{bmatrix} \begin{cases} \langle q_I(s) \rangle \\ q_C(s) \end{cases} + \begin{bmatrix} Ms^2 \end{bmatrix} \{q\} = \begin{cases} R_I(s) \\ \langle 0 \rangle \end{cases}$$

No interior load = dynamic condensation

$$[T(\omega)] = \begin{bmatrix} I \\ -Z_{CC}(\omega)^{-1}Z_{CI}(\omega) \end{bmatrix}$$

 $[T] = \begin{vmatrix} I \\ -K_{CC}^{-1}K_{CI} \end{vmatrix}$

Inertia neglected = static/Guyan



Frequency limit -> Craig-Bampton



Learning strategy variants : POD

Traditional : modes + static correction



Relatively close static correction

Learning strategy : wave/cyclic

- 1. Learn using wave (Floquet)/cyclic solutions
- 2. Build basis with left/right compatibility
- 3. Assemble reduced model

Mode 1 at 3.685 Hz Mode 2 at 6,496 Hz Mode 3 at 10:53 Hz













- 1. Learning phase
 - 1. modes & static responses (bandwidth, inputs) : McNeal, Guyan, Craig-Bampton
 - 2. POD
- 2. Basis generation DOF selection
 - 1. SVD (truncation)
 - 2. Gramm-Schmidt, conjugate-gradient (Lanczos)
 - 3. Piecewise learning (sparsity, superelements, Component mode synthesis)
- 3. Model reduction/modal synthesis/Ritz-Galerkin/virtual work principle $\{q(x,t)\} = [T(x)]\{q_R(t)\} \Rightarrow Z_R(\omega,p) = T^T[Z(\omega,p)]T$

Optimize reduced model usage

4. Beyond LTI (parametric, NL, time varying, ...)

T independent of p



{q}_N=

Tuto steps 3-4

- POD learning
- Rayleigh-Ritz / reduced solve

SVD & variants



SVD

- {X} on sphere in input space transformed in {Y}=[A]{X} ellipsoid
- Series of rank one contributions

Mode

 {φ} on unit strain energy sphere output is kinetic energy

• Singular value
$$\frac{1}{\omega_j^2} = \frac{\phi_j^T M \phi_j}{\phi_j^T K \phi_j} = 1/\text{Rayleigh quotient}$$



shape

DOF

Optimize reduced model computation

Spectral decomposition

$$\{y\} = c_R [M_R \ s^2 + K_R]^{-1} b_R u = \sum_{j=1}^{NM} \frac{c\phi_j \phi_j^T bu}{s^2 + 2\zeta_j \omega_j s + \omega_j^2}$$

- Cost is $O(N^3) \times N_F$ or $O(N^3) + O((NM \times N_a \times N_s) \times N_F)$

- The second is obviously much lower for not very small N_F
- Transition matrix (or matrix exponential) : time domain (free response), space domain (Wave Finite Element) $\{x_{n+1}\} = [A]\{x_n\} = \left[\Theta_R\left[\backslash\lambda_{j_n}\right]\Theta_L^T\right]\{x_n\} = \left[\Theta_R\left[\backslash\lambda_{j}^{n+1}\right]\Theta_L^T\right]\{x_0\}$
 - The second is obviously much lower for not very small n

SVD, variants, related

Random fields Karhunen-Loeve :

- input-norm I for all DOFs
- output norm spatial correlation $C = \exp[-(|x_1 - x_2| + |y_1 - y_2|)]$

PCA Principal Component Analysis POD based on snapshot-reduction :

- input-norm I on snapshot vectors
- output norm I

Junction modes

- input-norm I for modes or contact stiffness
- output norm local stiffness

Non-linear dimensionality reduction (manifold)

More complex relation between parameters



Chung, Gutiérrez, & all, "stochastic finite element models," IJME, 2005.
 Kershen & al. "POD", Nonlinear dynamics, 2005
 Balmes, Vermot, "Colloque assemblages 2015",
 Bendhia 1-epsilon compatibility EJCM 2010
 Ph.D. Olivier Vo Van 2016



From shapes to bases

Vector independence

- · SVD
- Krylov/Lanczos (iterations & conditioning, step5)
- Gram Schmidt O. Boiteau, « Modal Solvers and resolution of the generalized problem (GEP) », Code_Aster, Version 5.0, R5.01.01-C, p. 1-78, 2001.
- LU



Multi-frontal solvers / AMLS

- Graph partionning methods ⇒ group DOFs in an elimination tree with separate branches
- Block structure of reduction basis
- Block diagonal stiffness
- Very populated mass coupling
- Multi-frontal eigensolvers introduce some form of interface modes to limit size of mass coupling



Interface reduction / model size / sparsity

Craig-Bampton often sub-performant because of interfaces

 $2^{e}6 \text{ rest x } 5000 \text{ Int } = 7468$

- Unit motion can be redefined : interface modes Fourier, analytic polynomials, local eigenvalue 5000 -> 500 interface DOFs.
- Disjoint internal DOF subsets



Separate requirements for learning shapes & basis building :

bandwidth, inputs external & parameter truncation, sparsity

MATLAB Tutorial : reduction, full operators

- Step 5 : Krylov
- Step 6 : sparse reduced model

- Step 7 : frequency limit CB
- Step 8 : an experimental case of SVD

DOF / sensor selection

Solutions depends on subspace NOT basis Choose DOF you like or that make sense

Ex 1: beam shape functions

- Subspace $a + bx + cx^2 + dx^3$
- Observation $y = \{w_1, \theta_1, w_2, \theta_2\}^T$



• Condition of unit on observation gives shape functions N_i

Ex 2: multibody dynamics : use master nodes needed [c][T] full rank $\{y\} = [c][T]\{q_R\}$ \downarrow



Physical & Modal DOF

Physical domain:

 $[M]\{\ddot{q}(t)\} + [C]\{\dot{q}(t)\} + [K]\{q(t)\} = \{f(q, \dot{q}, t)\}$

• Modal domain:

- mass orthogonality condition $\phi^T M \phi = I$
- stiffness orthogonality condition $\phi_j^T K \phi_j = \omega_j^2$
- Modal equation

 $[I]\{\ddot{\alpha}(t)\} + [\Gamma]\{\dot{\alpha}(t)\} + \left[{}^{\backslash}\omega_{j\,\backslash}^2 \right] \{\alpha(t)\} = \{f(\alpha,\dot{\alpha},t)\}$

- Modal amplitudes $\{\alpha\} = [\phi^{-1}]\{q\} = [\phi^T M]\{q\}$ Associated concepts : force appropriation, modal filter
- Modal energies $e_j = \frac{1}{2} (\dot{\alpha}_j^2 + \omega_j^2 \alpha_j^2)$

^[1] J. P. Bianchi, E. Balmes, G. Vermot des Roches, et A. Bobillot, « Using modal damping for full model transient analysis. Application to pantograph/catenary vibration », in *ISMA*, Leuven, 2010.

Modal participations in ODS



- Extract shape =
 SVD around « resonance »
- Obtain modal amplitudes of nominal modes
- Apparent stiffness/damping consistent for various methods







Modal energy computations



[2] G. Vermot Des Roches et E. Balmes, « Understanding friction induced damping in bolted assemblies through explicit transient simulation », in *ISMA*, 2014, p. ID360.

Modal DOF

- Multi-stage cyclic symmetry (SNECMA).
 - Which stage, which diameter, ...
 - Mistuning (which blade)



(c)

Dealing with NL/parameters/damping



Viscoelastic constitutive relations

- Stress is a function of strain history
- Complex modulus in Laplace domain $\sigma(s) = E(s,T,\sigma_0)\varepsilon(s) = (E'+iE'')\varepsilon(s)$



• Dynamic stiffness linear combination of fixed matrices $Z(E_i, s) = \left[Ms^2 + K_e + \sum_j E_i(s, T, \sigma_0) \left[\frac{K_{vi}(E_0)}{E_0} \right] \right]$

Residue iteration : viscoelastic material

 $[Z(E_i(s), s)]{q} = {F}$ Damped viscoelastic resp. rewritten as

$$[Z(E_0, s)]{q} = \{F\} - \sum_i (E_i(s) - E_0) \left[\frac{K_{vi}(E_0)}{E_0}\right]{q}$$

Tangent linear system, internal NL/parametric loads

Basis contains

Modes to represent nominal resonances

•Flexibility to viscoelastic loads associated with nominal modes

$$T = \left[\oint_{1:NM} K_{o}^{-1} \left[Im(Z-Z_{o}) \right] \oint_{1:NM} \right]$$
Modes static response to parametric load

Principle of reduction (assumptions on excitation space & freq) unchanged

What does first order bring?

- Correct energy distribution
- Accuracy on peaks (modal is over-damped up to 100%)



First order shape : $T = [K_0^{-1} [Im(Z-Zo)] \phi_4]_{orth}$



Parametric loads & reduction

Space/time decomposition of load $[b_{contact}]_{N \times Ng} \{p(t)\}$

- Know nothing about ${p(t)}_{Ng}$ too large
- {p(t)} associated with initial modes = $[[c_{NOR}][\phi_{1:NM}]]_{N \times NM}$ { $q_r(t)$ } Static correction for pressure load of elastic normal modes $T = [\phi(p_0) \ K^{-1}[b_c c_{NOR}\phi(p_0)]_{N \times NM}]_{\perp}$
- Multi-model learning $T = [\phi(p_1) \phi(p_2)_{N \times NM}]_{\perp}$

Error control (residue iteration)

$$R_d = K_0^{-1} \left\{ [M_0 s^2 + K_0] \{ Tq_R \} - [b_{ext}] \{ u_{ext} \} + \left\{ f_p(Tq_R, p) \right\} \right\}$$

PhD A. Bobillot 2002





MATLAB/SDT TutoParametric

- Step 1 : Load model
- Step 2 : Multi-model reduction
- Step 3 : Analyze frequency/damping evolution
- Step 4 : Analyze MAC, use modal coordinates

Fixed basis : enormous cost reduction

- Windshield joint complex modes at 500 design points for ½ cost of direct solver
- Campbell diagram : 200 rotations speeds for the cost of 4.
- Squeal instabilities as function of pressure : few pressures sufficient for interpolation

Ψ,λ	SOL107	2200s
Φ,ω	SOL103	300s
Ψ,λ Reduced	First order Error <4%	490s
Ψ,λ(500*Τ)	SOL107	~ 12 days
$\Psi,\lambda(500*T)$ reduced	First order Error small	~1000s







Reduction / response surface / HBM-PGD

Fixed basis reanalysis

Response surface for system matrices
 T^TZ(p)T≈f(p,T^TM_iT)

But

- still dynamic model
- restitution {q}=[T]{q_R} provides estimates of all internal states
- Response surface/meta-model methodologies
- also predict I/O relation
- but no knowledge of internal state





<u>PGD & HBM methodologies</u> : variable separation of higher dim

$$\{q(t,p)\} = \sum_{i} \{T_{i \, space}\}\{T_{i \, time}\}q_i(p)$$

Conclusions : solvers for dynamics

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